Stanford County Department of Public Works Residential Soils Report Form

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SESSIONAL ENGLIS		
		CSS CONTRACTOR

Stafford County

Department of Public Works Residential Soils Report Form

3-	
BASED ON THE SOIL TEST RESULTS (CHECK APPROPRIATE):	
The foundation design indicated on the construction drawings meet/ exceed the requirements set forth in the Uniform Statewide Building Code and the International Residential Code 2006 edition. (designer/ contractor must complete checklist "A" and seal each applicable page of this document).	
Because the soil conditions are beyond the parameters outlined in the USBC/ IRC the proposed construction requires and engineered foundation design. (engineer must complete checklist "B" and soal cach applicable page of this document.	
CHECKLIST A	
THIS SECTION MUST BE COMPLETED WHEN CONSTRUCTION PLANS MEET OR EXCEED THE	

PRESCRIPTIVE CODE REQUIREMENTS AS RELATED TO THE FINDINGS OF THE SOILS ANALYSIS. THE CONSTRUCTION PLANS MUST ACCOUNT FOR ALL CONSIDERATIONS IN THE CHECKLIST BELOW. (CHECK WHERE APPROPRIATE): WALL CONSTRUCTION DESIGN LOAD BEARING FOR: REINFORCEMENT PERIMETER FOUNDATION FOOTINGS FOOTING WIDTH SLAB FOOTING THICKNESS INTERMEDIATE AND PIER FOOTINGS PROJECTION WALL THICKNESS PIER SIZE (FOR LATERAL EARTH PRESSURE AND UNBALANCED SOIL LOAD) PIER HEIGHT FOOTING DEPTH SLAB THICKNESS FOOTING WIDTH FROST DEPTH MONOLITHIC/ TURN DOWN SLAB DESIGN . DESIGN FOR USBC 602,10.6 IT IS IMPORTANT THAT WHEN USING THIS OPTION THAT ALL NOTES AND DETAILS ON THE CONSTRUCTION DRAWINGS CLEARLY CORRESPOND WITH SOILS ANALYSIS AS THEY RELATE TO USBC/IRC REQUIREMENTS. REQUIREMENTS WITHIN THE IRC/ USBC ARE NOT LIMITED TO THE ABOVE LISTED CRITERIA.

I HAVE REVIEWED THE CONSTRUCTION DRAWINGS AND FIND THAT THEY SATISFY THE REQUIREMENTS WITHIN THE VIRGINIA UNIFORM STATEWIDE BUILDING CODE.

DESIGNER/ CONTRACTOR:

Stafford County
Department of Public Works
Residential Soils Report Form

CHECKLIST B

	ENGINEERED FOUNDATION IS REQUIRED. IT IS ION SYSTEM ACCOUNTS FOR ALL OF THE LISTED
CRITERIA BELOW.	
THE ENGINEERED FOUNDATION SYSTEM IS:	INCORPORATED IN THE CONSTRUCTION DRAW
-	DETAILED AS AN ATTACHMENT

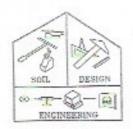
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alling of this document certifies that all information submitte record performed/ supervised all soll sampling, testing, eval	d is accurate and that the engineer lustion and execution of this report.
ealing of this document certifies that all information submitte	ENGINEERS SEA JOE F, ALEXAN 31/2/M. 8458

Stafford County

Department of Public Works Residential Soils Report Form

Certain soil types and conditions are not outlined in the International Residential Code (non-prescriptive). If native (on site) soils are deemed unsuitable for backfill against foundation walls or for footing support the contractor, in coordination with the soils engineer, may choose to utilize borrowed or engineered fill material. When this is the case, a registered design professional shall oversee and certify all borrowed or engineered fill. This form must be completed, sealed and on site for inspection prior to final inspection. Please check all applicable items.

material. When this is the case, a registered design professional shall oversee engineered fill. This form must be completed, sealed and on site for inspection Please check all applicable items.	e and certify all borrowed or on prior to final inspection.
The proposed construction is to be placed on a fill pad. (Please attach ap- certification with this form).	proved engineered pad
The proposed construction is on a site composed of expansive soils.	
The proposed construction is on a site composed of soils with bearing ca is specified in the construction drawings.	apability less than that which
Other condition that can not satisfy the requirements within the IRC Interpretation (please explain):	ernational Residential Code
Scaling this document certifies that all soil on site is suitable for the propose the construction drawings. This includes all undisturbed soil to support footices.	d construction as specified on ngs, backfill against
foundation walls, borrowed fill and fill sites.	ENGINEERS SEAL



AlexCom & Associates, Inc.

11 Debruen Lane Fredericksburg, Virginia 22405 Tel; (540) 371-3157 Fax: (540) 371-7920

Email: alexcom@vacoxmail.com

March 1, 2016

Casablanca Enterprises 1033 Isabella Drive Stafford, VA. 22554

Reference: 306 Ironside Cove

Shrink Swell Soil Investigation Stafford County, Virginia AlexCom Project # 150830

To Whom It May Concern:

Purpose:

As per your request, AlexCom & Associates performed a shrink/swell soil investigation of the above referenced lot in Stafford County, Virginia. The investigation was conducted in a manner that would allow for determination of problematic soil conditions such as bearing capacity, high water table indicators, and potential shrink/swell soils.

Procedure:

The investigation, which was performed in accordance with the Stafford County Soil Testing Policy, included the evaluation of auger borings at opposite corners of the proposed structure. The locations were determined from client provided stakes or approximated from available information utilizing topographical interpretation along with taping and pacing techniques.

Based upon a client provided site plan or typical construction practice, the borings were extended to a depth of at least two feet (2') beyond anticipated footing elevation unless auger refusal occurred. Encountered soil horizons were visually categorized and logged utilizing the Unified Soils Classification System (USCS) with additional notes regarding any soft, moist, or unsuitable soils. The presence and depth of encountered subsurface water was estimated during excavation. Unless denoted by a laboratory number, the soil horizon descriptions and classifications contained in the boring logs are based upon visual observation by a Soil Technician. The soil bearing capacity was determined as the boring was advanced based upon auger resistance and use of a "T" bar probe when required.

A sample of the soil deemed to have the greatest potential for expansive behavior at or below footing depth was taken for laboratory analysis of its grain size distribution, Atterberg Limits, and linear shrinkage. The shrinkage test was performed in accordance with ASTM D427 and the USCS Classification was based on Atterberg Limits conducted in accordance with ASTM D4318 and ASTM D2487.

Limitations / Conditions:

The results of this investigation may be substantially influenced by the accuracy of provided information. In situations where a site plan is not provided and/or the house corners have not been staked, it should be understood that it is the client's responsibility to contact AlexCom if the field conditions differ from those presented in this investigation. Groundwater elevations are highly variable and it is possible that the elevation or presence of moisture seepage encountered during excavation may vary from those indicated in this report. The results and recommendations provided in this report do not reflect variations in subsurface conditions or in unexplored portions of the site. If changes to the location, elevation, or foundation type of the home occur after the initial data for this investigation has been obtained, then the results should not be considered valid until AlexCom & Associates has been provided an opportunity to review the changes and, if necessary, modify the recommendations.

Sincerely yours,

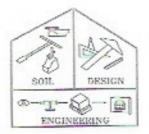
Ioe F. Alexander, PE

Geotechnical Engineer

ENC: Stafford County Shrink/Swell Soil Form, Boring Logs, Laboratory Results, Foundation Design

CC: File

File Path: P:\Geotech\2016\2016\Shrink Swelf\306 Ironside Cove - Casablanca Enterprises - Stafford Co. Letter.doc



AlexCom & Associates, Inc.

Fredericksburg, Virginia 22405 Tel: (540) 371-3157 Fax: (540) 371-7920 www.alexcomva.com

Professional Memberships: ICC, WACEL, NOWRA, VOWRA, AMRL

March 2, 2016

Casablanca Enterprises 1033 Isabella Drive Stafford, Va. 22554

Reference: 306 Ironside Cove; Stafford Co. VA pH Test Results

The pH of the soil was tested with the PH55 Waterproof Meter as referenced in Milwaukee PH55 Waterproof Meter Manual.

The pH level for Bore Log Sample #2 was determined to be 5.1. For grass growth purposes, the area should be treated with 150lbs of lime per 1000 sq/ft of area to promote grass growth and correct levels of pH to 6.5.

The pH level for Bore Log Sample #4 was determined to be 5.0. For grass growth purposes, the area should be treated with 140lbs of lime per 1000 sq/ft of area to promote grass growth and correct pH level of acid soils to 6.5

Lic. No. 01583

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Results:

Summarized findings of the borings and laboratory tests are presented in the underlying table with detailed results attached to this report:

Summarized Investigation Results B-2 Bore Log

Recommended Soil Bearing Capacity	2,000 psf
Fill Encountered	No
Groundwater Indicators Encountered	No
Auger Refusal Encountered	No
Sample Classification	MH
pH	5.1
Liquid Limit	52.5
Plasticity Index	17.8
Natural Moisture Content	27.9%
Linear Shrinkage	19.7%
Design Lateral Soil Load	*Note
Special Foundation Design Required	Yes

Summarized Investigation Results B-4 Bore Log

Recommended Soil Bearing Capacity	1,500 psf
Fill Encountered	No
Groundwater Indicators Encountered	No
Auger Refusal Encountered	No
Sample Classification	ML
pH	5.0
Liquid Limit	38.2
Plasticity Index	10.3
Natural Moisture Content	14.0%
Linear Shrinkage	8.3%
Design Lateral Soil Load	45lb
Special Foundation Design Required	No

Recommendations:

Based upon the findings of the investigation, it appears that the soil horizons encountered within the zone of influence for the foundation exhibit a moderate to high potential for shrink/swell behavior. A special foundation design will be required to resist the lateral soil pressures and minimize potential subgrade expansion/contraction due to seasonal variations in moisture content. *Note: Sample B-2 is MH soil and is NOT suitable for backfill. Use #57 stone or approved off site material for backfill purposes.

ALEXCOM & ASSOCIATES, INC SHRINK / SWELL REPORT

Project Na	me:		306 Irons	side Cove		Project No	imber: 150830
Client Nan	ne:		Bill	Roth		Date:	November 24, 2015
Section:		Phas	e:	Lot:		County:	Stafford
Lab No.	Sample	Depth	USCS			Description	
3499 - 15	2	48"	MH			Elastic Silt	
3499 15	4	48"	ML			Silt	
PROPERT	ГҮ		3499 - 1	5 3499	15		
Chainlean	I don't .		C 50/	20.0	207		

PROPERTY	3499 - 15	3499 15		
Shrinkage Limit :	-6.5%	22.9%		
Shrinkage Ratio:	1.6	1.5		
Volumetric Shrinkage:	93%	30%		
Linear Shrinkage:	19.7%	8.3%		
Specific Gravity:	1.46	2.30		

SHRINKAGE LIMIT: The shrinkage limit of a soil is the maximum water content at which a reduction in water will not cause a decrease in volume of soil mass.

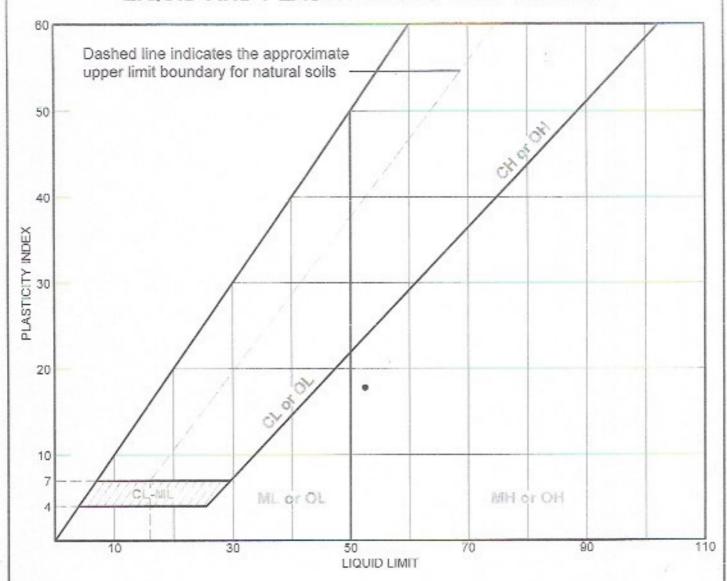
SHRINKAGE RATIO: The shrinkage ratio of a soil is the ratio of a given volume change expressed as a percent of the dry volume relative to the corresponding change in water content above the shrinkage limit, expressed as a percentage of the mass of oven dried soil

VOLUMETRIC SHRINKAGE: The volumetric shrinkage of the soil is the decrease in volume, expressed as a percentage of the soil mass when dried, of a soil mass when the water content is reduced from a given value to the shrinkage limit.

LINEAR SHRINKAGE: The linear shrinkage of a soil is the decrease in one dimension of a soil mass, expressed as a percentage of the original dimension when the water content is reduced from a given value to the shrinkage limit.

SPECIFIC GRAVITY: The specific gravity of a soil is the weight in air of a given volume of soil particles at a stated temperature to the weight in air of an equal volume of water at the stated temperature.

LIQUID AND PLASTIC LIMITS TEST REPORT



SOIL DATA								
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	USCS
•	3499-15	2	48"	27.9	34.7	52.5	17.8	МН

LIQUID AND PLASTIC LIMITS TEST REPORT

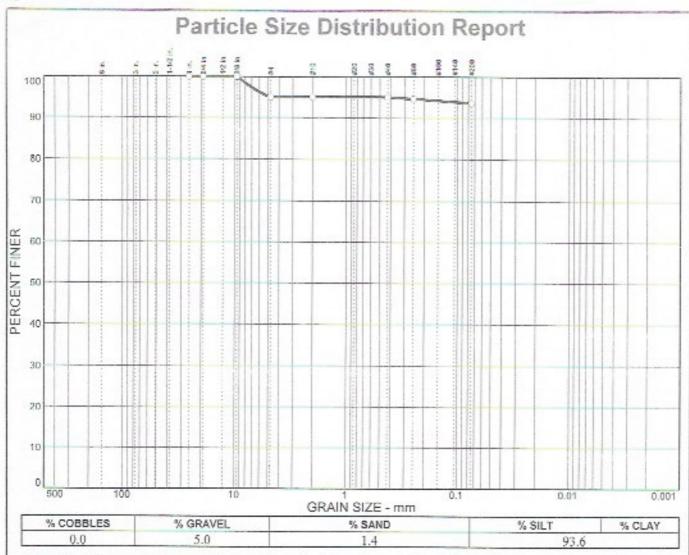
AlexCom & Associates, Inc. Consulting Engineers

Phone: 540-371-3157 * Fax: 540-371-7920

Client: Bill Roth

Project: 306 Ironside Cove

Project No.: 150830



SIEVE	PERCENT	SPEC.* PERCENT	PASS? (X=NO)
1 in. .75 in. .375 in. #4 #10 #40 #60 #200	100.0 100.0 100.0 95.0 95.0 95.0 94.6 93.6		

Elastic silt	Soil Descriptio	<u>n</u>
PL= 34.7	Atterberg Limit	<u>s</u> Pl= 17.8
D ₈₅ = D ₃₀ = C _u =	Coefficients D ₆₀ = D ₁₅ = C _c =	D ₅₀ = D ₁₀ =
USCS= MH	Classification AASH	TO= A-7-5(21)
	Remarks	

(no specification provided)

Sample No.: 2

Source of Sample: 3499-15

Date: 11/24/15

Location: Back Right Corner

Elev./Depth: 48"

AlexCom & Associates, Inc.

Consulting Engineers

Client: Bill Roth

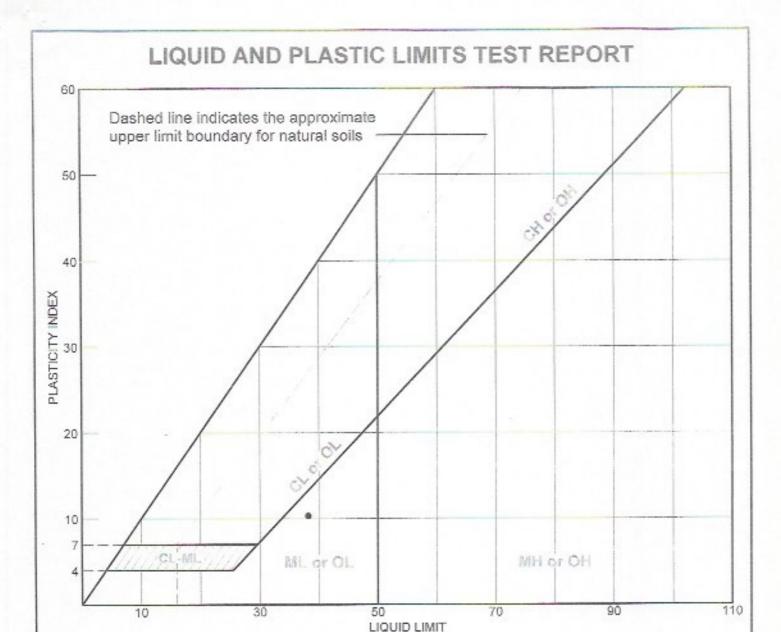
Project: 306 Ironside Cove

Phone: 540-371-3157 * Fax: 540-371-7920

Project No: 150830

BORING LOG BORE HOLE No.: B-2

PROJECT							PROJECT NO.			
306 Ironside Cove							150830			
CLIENT								DATE		
Roth, Bill								11/24/	15	
LOCATION								ELEV.		
Left Front Corner								100'as		
EXCAVA	LOGGER DH									
DEPTH 7	Hand auger DEPTH TO - Water: n/a When checked: 11/24/15 Caving:									
ELEVATION/ DEPTH	SOIL SYMBOLS AND SAMPLERS	uscs	LIQUID		LINEAR SHRINKAGE	DESCRIPTION		SPECIFIC GRAVITY	NATURAL MOISTUR	
0 _ 00	[22222]	TPS	-			Top soil				
98 - 2	[v]	МН				Tan Reddish Grey Sandy S DCP @ 4.5', 12-12-15, 20 psf.	ilt, 000			
96 - 4										
94 - 6										
90 - 10										
-									† -	
ALL SOIL	CLASSIFICATIO	NS WE				ILESS DENOTED BY A LA	ABORA	ATORY N	UMBER	



				SOIL DATA				
SYMBOL	SOURCE	SAMPLE NO.	DEPTH (ft.)	NATURAL WATER CONTENT (%)	PLASTIC LIMIT (%)	LIQUID LIMIT (%)	PLASTICITY INDEX (%)	uscs
٠	3499-15	4	48"	14.0	27.9	38.2	10.3	ML

LIQUID AND PLASTIC LIMITS TEST REPORT

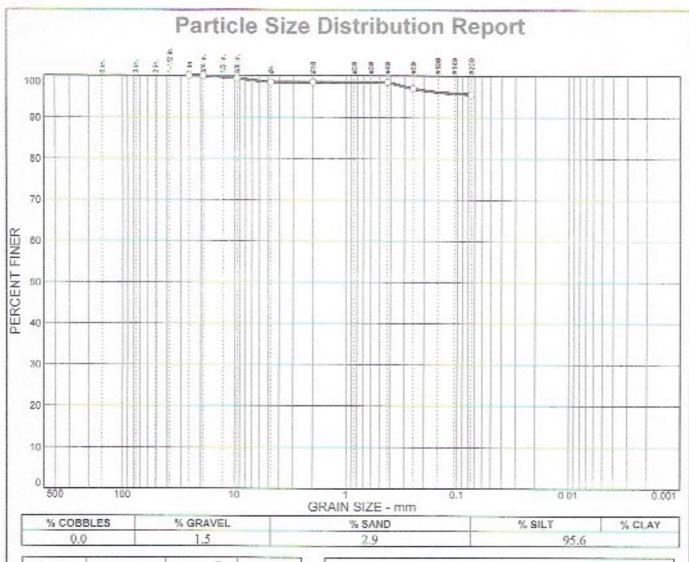
AlexCom & Associates, Inc. Consulting Engineers

Phone: 540-371-3157 * Fax: 540-371-7920

Client: Bill Roth

Project: 306 Ironside Cove

Project No.: 150830



		SPEC.*	PASS?
IZE	FINER	PERCENT	(X=NO)
1 in. 5 in. 5 in. #1 #10 #40 #60 \$200	100.0 100.0 99.4 98.5 98.5 98.5 97.0 95.6		

Silt	Soil Description	
PL= 27.9	Atterberg Limits LL= 38.2	PI= 10.3
D ₈₅ = D ₃₀ = C _u =	Coefficients D ₆₀ = D ₁₅ = C _c =	D ₅₀ = D ₁₀ =
USCS= ML	Classification AASHT	0=
	Remarks	

* (no specification provided)

Sample No.: 4

Source of Sample: 3499-15

Date: 11/24/15

Location: Front Left Corner

Elev./Depth: 48"

AlexCom & Associates, Inc.

Client: Bill Roth

Consulting Engineers

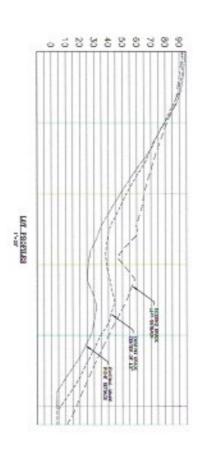
Project: 306 Ironside Cove

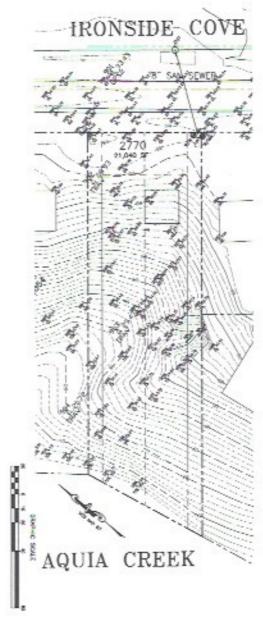
Phone: 540-371-3157 * Fax: 540-371-7920

Project No: 150830

BORING LOG BORE HOLE No.: B-4

PROJECT 306 Ironside Cove							PROJECT NO. 150830				
CLIENT Roth, Bill								DATE 11/24/15			
LOCATION								ELEV.			
	Back Left Corner EXCAVATION METHOD Hand auger DEPTH TO - Water: n/a When checked: 11/24/15 Caving:								100'as LOGGER		
EXCAVA											
DEPTH 1									DH g: n/a		
ELEVATION/ DEPTH	SOIL SYMBOLS AND SAMPLERS	USCS	LIQUID	PLASTIC INDEX	LINEAR SHRINKAGE	DESCRIPTION		SPECIFIC GRAVITY	NATURAL MOISTURE %		
100 — 0	[22222]	TPS				Topsoil					
98 — 2		ML				Tan Sandy Silt, DCP @ 4', 10, 1500 psf.	8-9-	-	-		
96 - 4									-		
94 6									_		
20 10											
90 + 10									- - - -		
ALL SOIL O	CLASSIFICATION	IS WE				LESS DENOTED BY A LAI	BORA	TORY N	UMBER.		





PLAN AND PROFILE

306 IRONSIDE COVE

FOR DILL RODI

Aquile Megisterial District. Stafford County. W.



ALEXCOM & ASSOCIATES, INC.

GROTINCHINGAL / CIVIL AND
STRUCTURAL REGINERAS
11 DARFOLD LONG
11 DARFOLD LONG
PROCEEDINGS (STORY 25-02)
PROCE (\$400 \$711-3187, Day (\$400 \$711-7920)

